MSSA-PBK, P.O. BOX 880, GRIFFIN, GA 30223

ADDENDUM NUMBER ONE: DATED: February 10, 2025

To: All prime contract bidders of record.

This addendum forms a part of the Contract Documents and modifies the original Specifications and Drawings dated November 11, 2024, as noted below. **Acknowledge receipt of this Addendum in the space provided on the Bid Form;** failure to do so may subject bidder to disqualification.

BID DATE: February 27, 2025

SPECIFICATIONS

1. Section 003132A – GEOTECHNICAL DATA
   A. Add Section 003132A – Geotechnical Data included in this Addendum.
2. Section 102800 – TOILET, BATH, AND LAUNDRY ACCESSORIES
   A. On Page 102800-5, delete paragraph 2.4.B in its entirety and replace with the following:

      "B. Warm-Air Dryer (Hand Dryer) <**TA-14**> :

      1. Basis-of-Design Product: Model Q-974A2 Verde dri.
      2. Mounting: Surface mounted.
      3. Operation: Non-touch infrared activated with timed power cut-off switch.
         a. Operation Time: 12 second run on and 45 second vandal resistant lock out feature.
      4. Cover Material and Finish: One piece, vandal resistant, with Cast Aluminum with White finish.
      5. Electrical Requirements: Coordinate with existing or as indicated on electrical documents.

DRAWINGS

1. Sheet A6.11, Misc. Details
   A. Delete Sheet A6.11 in its entirety and replace with Sheet A6.11, dated 02/10/25, included in this Addendum.

DOCUMENT 003132 - GEOTECHNICAL DATA

1.1 GEOTECHNICAL DATA

A. This Document with its referenced attachments is part of the Procurement and Contracting Requirements for Project. They provide Owner's information for Bidders' convenience and are intended to supplement rather than serve in lieu of Offeror's own investigations. They are made available for Bidders' convenience and information, but are not a warranty of existing conditions. This Document and its attachments are not part of the Contract Documents.

B. A geotechnical investigation report for Project, prepared by Atlas Technical Consultants LLC dated October 7, 2024, is available for viewing as appended to this Document.

C. Related Requirements:

1. Document 002113 "Instructions to Offerors" for the Bidder's responsibilities for examination of the Geotechnical Data.

END OF DOCUMENT 003132

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SUBSURFACE EXPLORATION AND GEOTECHNICAL ENGINEERING EVALUATION

Fayette County High School Auxiliary Gym
Fayetteville, Georgia
Atlas Project No. 18889

PREPARED FOR:
Integrated Science & Engineering, Inc.
1039 Sullivan Road, Suite 200
Newnan, Georgia 30265

PREPARED BY:
Atlas Technical Consultants LLC
3000 Northfield Place, Suite 1100
Roswell, Georgia 30076
770-752-9205 * FAX 770-752-0890

October 7, 2024

ATLAS Project No. 18889

October 7, 2024

Integrated Science & Engineering, Inc.
1039 Sullivan Road, Suite 200
Newnan, Georgia 30265

Attention : Mr. Michael R. Madison, APWA CSM

Subject: **Report of Subsurface Exploration and**
Geotechnical Engineering Evaluation
Fayette County High School Auxiliary Gym
1 Tiger Trail
Fayetteville, Georgia

Dear Michael:

Atlas Technical Consultants LLC (Atlas) is pleased to provide this report of our subsurface exploration and geotechnical engineering evaluation for the referenced project. The field study and this report were accomplished in general accordance with Atlas Proposal No. 24-07770, dated July 26, 2024.

The following report will present a summary of our pertinent findings and recommendations followed by our understanding of the proposed construction, methods of exploration employed, site and subsurface conditions encountered, and conclusions and recommendations regarding the geotechnical aspects of the project. Should you have any question regarding items discussed in this report, please do not hesitate to contact the undersigned.

Sincerely,
Atlas Technical Consultants LLC

W Michael Ballard

W. Michael Ballard, P.E.
Chief Engineer

GEORGIA
REGISTERED
No. 17513
PROFESSIONAL
ENGINEER
W. MICHAEL BALLARD

WMB/ew
Attachments

TABLE OF CONTENTS

APPENDIX

1.0 SUMMARY

The following is a summary of our pertinent findings and recommendations. The reader is referred to the remaining text of this report for elaboration on these items.

1. General subsurface conditions consist of previously placed fill, underlain in places by alluvial soils deposited by the adjacent stream. Some organics were noted in the samples recovered. At our boring locations partially weathered rock and rock was found more than 30 feet below the existing grade.

2. Portions of the structure are underlain by what is either fill with some organics or more likely alluvium with some organics. As such we recommend that the load bearing elements of the building be supported on aggregate piers/stone columns.

3. Most of the on-site existing fills and residual soils encountered in our borings are visually suitable for reuse as structural fill. Several samples were recovered which included organics and soils with significant organic content should not be used in structural areas.

4. Excavations to the depths explored and at our boring locations can be accomplished using conventional heavy earthmoving equipment.

5. We request we be notified if any of the assumptions presented in report section 2.0 Proposed Construction are incorrect.

2.0 PROPOSED CONSTRUCTION

We understand that you are planning for design and construction of an Auxiliary Gymnasium at Fayette County High School. The Aux Gym will be constructed to the south and west of the existing gym and to the north of an existing stream. We anticipate the Aux Gym will be a 2-story structure and will abut but not be structurally connected to the existing gym. We have assumed maximum column loads will not exceed 150 kips. As a basis of this report, we believe the existing gym and new Aux Gym will have matching finish floor elevations. Thus, the building footprint will be raised from a negligible amount to as much as 10 feet to reach the FFE. If these assumptions are not correct, we request that we be notified as that may impact our recommendations.

No other details of the proposed construction were available at the time this report was prepared.

3.0 METHODS OF EXPLORATION

To evaluate the subsurface conditions, the property was explored by a combination of a visual site reconnaissance and drilling five (5) soil test borings to depths ranging from 35 to 50 feet below the existing grade. The borings were located in the field by measuring distances and estimating directions from identifiable site features (or by using GPS coordinates from a handheld GPS device). Therefore, their locations as shown on the Site and Boring Location Plan in the Appendix should be considered approximate.

The borings were advanced by twisting continuous hollow stem auger flights into the ground. At selected intervals, Standard Penetration Resistance Testing (SPT) was performed in general accordance with ASTM

Standard D-1586, and soil samples were collected for visual classification. The results of the penetration tests, when properly evaluated, provide an indication of the relative consistency of the soil being sampled, the potential for difficult excavation, and the soil's ability to support loads. A more detailed description of the drilling and sampling process is included in the Appendix of this report.

Soil samples recovered during the drilling process were returned to the office where they were classified in general accordance with the Unified Soil Classification System (USCS). Detailed descriptions of the materials encountered at each boring location, along with a graphical representation of the Standard Penetration Test results, are shown on the Soil Boring Records in the Appendix. Elevations on the Soil Boring Records were interpolated from the topographical contours on the plan provided to us and should be considered approximate.

4.0 SITE DESCRIPTION, GEOLOGY AND SUBSURFACE CONDITIONS

4.1 Site Description

The construction will take place to the southwest of the existing gym in an area currently covered with pavements and grass. To the south of the proposed Aux Gym is an existing creek. Within the construction area grades drop from the existing gym toward the creek with about 15 feet of topographic relief estimated. The southern Aux Gym building line is estimated to be about 5 to 15 feet above the elevation of the nearby creek bank, with the southwest portion being lowest in elevation. The area was observed on Google Earth to have been graded in about 1998 for the gym and new roadways.

4.2 Geology

The site is located in the Piedmont Physiographic Province of Georgia. The residual soils in the Piedmont are the result of the chemical and physical weathering of the underlying parent rock. The weathering profile usually results in fine grained clayey silts and silty clays near the surface, where weathering is more advanced. With depth, sandy silts and silty sands are found, often containing mica. Below the residual soils, partially weathered rock is often found as a transition above relatively unweathered rock. In local practice, partially weathered rock is arbitrarily defined as residual soils with Standard Penetration Resistances in excess of 100 blows per foot (50 blows per 6 inches), and which can be penetrated by a power auger.

4.3 Subsurface Conditions

4.3.1 Surface Conditions

All borings initially encountered 2 to 6 inches of topsoil and root zone. Not encountered by the borings but surface conditions also consist of pavement, sidewalks, and equipment pads.

4.3.2 Previously Placed Fill

Fill soils are those soils that have been placed or reworked in conjunction with past construction activities, grading, or farming. All borings encountered previously placed fill below the topsoil extending to depths which ranged from 3 to 15 feet. The fill was typically classified as dense to medium dense silty sand or stiff to very stiff sandy and clayey silt, with Standard Penetration Test (SPT) results ranging from 11 to 35 blows per foot (bpf). Based on the SPT results, the soil represented by these samples would be considered well compacted.

4.3.3 Alluvium

Alluvium is soil that has been transported and deposited by moving water. Below the fills, several of the borings encountered soils with the physical appearance of alluvium to depths ranging from 12 to 17 feet. The samples recovered in boring B-2 included significant organics and in B-5 included trace organics.

4.3.4 Residuum

Residuum, formed by in-place weathering of the parent rock, was encountered below the fill or alluvium in each boring. The initial contact with residual soils varied from8 to 17 feet below the existing grade. The residuum was classified as loose to dense silty sands and firm to very still sandy and clayey silts and was of moderate consistency. Standard Penetration Test results ranged from 6 to 40 blows per foot, with 8 to 15 bpf being typical.

4.3.5 Partially Weathered Rock

Partially weathered rock (PWR) is a transitional material between soil and rock, which retains the relic structure of the rock and has very hard or very dense consistencies. All borings encountered partially weathered rock with the initial contact at depths which ranged from 32 to 42 feet below the existing grades. The PWR was classified as very dense silty sands.

4.3.6 Drilling Refusal (Presumed Rock)

Refusal is a designation applied to any material which cannot be further penetrated by the power auger and is normally indicative of a very hard or very dense material, such as boulders or lenses or the upper surface of bedrock.

Refusal to the drilling process was encountered in all borings except B-3 which was terminated in PWR at its planned depth of 50 feet. The remaining borings encountered drilling refusal at depths which ranged from 33.5 to 46.5 feet below the existing grades. The nature of the refusal material can only be determined by advancing the hole below the refusal depth by using rock coring techniques, which were beyond the scope of this work.

4.3.7 Groundwater

Groundwater was encountered at the time of drilling at depths ranging from 13 to 18 feet below the existing grade. For student safety, all boreholes were backfilled immediately upon completion of drilling and stabilized groundwater levels were not measured. We expect stabilized (24 hour) groundwater levels to be higher than the those measured at the time of drilling. Groundwater fluctuations of 5 feet or more are common in this geology.

The conditions described in the preceding paragraphs, and those shown in the Appendix, have been based on interpolation of the results of the previously described data using generally accepted principles and practices of geotechnical engineering. However, conditions in this geology may vary intermediate of the tested locations, and even more so on previously developed property.

Although individual soil test borings are representative of the subsurface conditions at the precise boring locations on the day drilled, they are not necessarily indicative of the subsurface conditions at other locations or other times. The nature and extent of variation between the borings may not become evident until the course of construction. If such variations are then noted, it will be necessary to reevaluate the recommendations of this report after on-site observation of the conditions.

5.0 CONCLUSIONS AND RECOMMENDATIONS

The following conclusions and recommendations are based on the data gathered during this exploration, our understanding of the proposed construction, our experience with similar site and subsurface conditions and generally accepted principles and practices of geotechnical engineering. Should the proposed construction change significantly from that described in this report, we request that we be advised so that we may amend these recommendations accordingly. This report and the conclusions and recommendations provided herein are provided exclusively for the use of Integrated Science & Engineering, Inc. and are intended solely for design of the referenced project.

5.1 General

This property has been previously graded, and, to the best of our knowledge, there are no records documenting inspections or testing that took place during the grading operation. Since no documentation of the overall grading process is available, there is the possibility that undetected areas of unsuitable soils were left in place or were placed during the grading, that pits have been dug and unsuitable soils or organics have been buried, or other unanticipated conditions may exist. This is a risk inherent in development on an undocumented, previously graded property.

5.2 Site Preparation

As an initial step in site preparation, all trees and unwanted vegetation should be removed, stumps grubbed, and organic topsoil stripped. Asphalt and concrete pavement should be removed but the underlying base stone can remain in place. Equipment pads should be removed. There are several underground utilities that cross the project area. Where these are relocated the excavations should be backfilled in accordance with this report. Where utilities are to remain in place those backfills should be evaluated by proofrolling as described in the paragraph below. If those backfills are found to be unstable, they should be remediated in accordance with the recommendations of the geotechnical engineer present during the construction.

All areas to receive fill and the surface conditions of all underground utilities that will remain should be evaluated prior to fill placement. The approval process should include proofrolling the subgrade with a fully loaded tandem axle dump truck (20 tons) during a period of dry weather and under the observation of the geotechnical engineer. Any areas which "pump" or "rut" excessively under the weight of the proofrolling vehicle should be further evaluated and may require undercutting or other remediation. The proofrolling can occasionally detect pits where stumps or other debris may have been buried, or other areas where weak surface conditions exist.

5.3 Earthwork

The majority of the existing fills and residual soils encountered in our borings visually appear suitable for reuse as structural fill. Alluvial soils are not expected to be suitable for structural use if any are excavated during utility construction. Soils which include heavy organics should not be used in structural areas. Moisture control may be necessary.

It is our opinion that the on-site soils are generally suitable for use as structural fill. However, use of Piedmont Geology residual soils as backfill may not be accepted by all designers of mechanically stabilized earth walls (modular block walls) if any are proposed for this site. While these soils generally meet weight and strength characteristics used by most retaining wall designers for soil backfill, they may not meet the gradation criteria for all wall designers. One nationally accepted retaining wall design standard does not permit use of soil as backfill which have more than 35% of their volume classified as clays or silts. There are local wall designers who recognize the difficulty of meeting that gradation criterion in the Piedmont Geology and who are willing to accommodate these soil types in their design. Without knowledge of which retaining wall designer may be selected for the project we can only advise you that some local designers will severely restrict “their definition” of suitable soils for retaining wall backfill. In those cases, the on-site soils may not be judged suitable by them for their purpose. As such, we recommend the retaining wall contractor discuss their ability to utilize the on-site soils as described on the boring records in the Appendix of this report prior to their selection.

All structural fill should be compacted to at least 95 percent of the soil's standard Proctor maximum dry density, as determined by ASTM Standard D-698. The upper foot of fill which will support pavements or slabs should be compacted to at least 98 percent of the soil's standard Proctor maximum dry density for improved support. In areas which are at or above the finished grade, and which will support pavements or slabs, the upper 8 inches immediately below these systems should be scarified and recompacted to the 98 percent criteria. Structural fill should be free of organic material, have a plasticity index (PI) less than 20 and contain rock sizes no larger than 4 inches.

Density testing should be performed by a soils technician to determine the degree of compaction and verify compliance with the project specifications. For underfloor areas, at least one field density test should be made per 5000 square feet of fill area for each two-foot lift. Testing frequency should be increased in confined areas. Areas which do not meet the compaction specifications should be recompacted to achieve compliance. In confined areas, such as utility trenches, the use of portable compaction equipment and thin lifts of 3 to 4 inches may be required to achieve compaction.

Excavations (to the depths and at the locations explored) can be accomplished using conventional heavy earthmoving equipment such as dozer assisted pans. No grading plan is available to see if all areas of deep excavations have been explored.

5.4 Foundations

The Auxiliary Gymnasium will be constructed over previously placed fills, some of which were placed over alluvial soils deposited prior to the past grading by the nearby stream. Some of the alluvium, or possibly the initial lifts of fill, includes organics which increases risk if those soils provide direct support for load bearing elements (column and wall footings). As such we recommend the building load bearing elements be supported on aggregate piers/stone columns.

Stone columns/aggregate piers are a viable mid-depth ground improvement system that should be designed by engineers experienced with design of those systems, usually under contract to the specialty foundation contractor. The following provides general guidance for use of these types of systems, but specifics need to be provided by the appropriate specialty contractor/designer experienced in the local area.

We believe that properly designed and installed aggregate piers/stone columns are suitable to improve subgrade conditions and to provide direct support to the structure footings. A conventional shallow foundation system bearing directly on a properly designed and installed aggregate pier/stone column system may be designed using allowable bearing pressures in the range of 5,000 to 6,000 psf. The actual bearing pressure should be determined by a licensed "aggregate pier or other" professional.

Stone column/aggregate piers are a proprietary ground improvement system consisting of medium-depth foundation elements composed of lifts of compacted stone typically installed in groups beneath shallow foundations. They are primarily used to provide support in weak or marginal soils and improve bearing conditions beneath shallow foundations or structural slabs. The stone column elements are columnar, typically range in depth from 10 feet to 25 feet and are composed of a series of 12-inch lifts of compacted aggregate. Installation is initiated by first drilling a shaft using a soil auger. Straight shafts 30 inches in diameter are typically specified. Once the shaft is drilled, a layer of open graded stone is placed in the bottom of the shaft. This layer of stone is forced into the soil forming a bottom bulb of stone at the base of the stone column. Piers are then completed by placing well-graded stone tamped in thin lifts. A licensed design professional experienced in these systems should select the actual bearing pressure to be used and from that will design pier spacing and location.

Once stone columns/aggregate piers are installed, no excavation should be done within 5 feet from the edge of the installed stone column. Thus, installation of utilities, any needed slab subgrade remediation or other excavations planned near foundation elements needs to be coordinated and accomplished prior to stone column installation.

5.5 Soil Supported Slabs

Floor slabs may be soil supported, subject to the subgrade preparation and earthwork recommendations contained in this report. Crushed stone is not needed to support the slab loads and is considered optional.

5.6 Temporary and Permanent Slopes

Permanent and temporary slopes may be used to accommodate grade changes. If temporary slopes are used, they should be constructed no steeper than 1.5H: 1V for slopes less than 15 feet high. Permanent slopes should be constructed no steeper than 2H: 1V. These recommendations are based on our experience with similar conditions and no detailed slope stability analyses have been performed. All finished slopes should be suitably protected from erosion. Buildings should be set back at least 10 feet from the top of slopes; a minimum 5-foot setback is considered sufficient for pavement areas.

5.7 Seismic Criteria for Structural Engineer

Based on the data collected from the site, the following structures should be designed using the Seismic Site Class “D” as determined by the International Building Code 2018/ASCE 7-16.

6.0 QUALIFICATIONS OF RECOMMENDATIONS

This evaluation of the geotechnical aspects of the proposed design and construction has been based on our understanding of the project and the data obtained during this study. The general subsurface conditions used in our evaluation were based on interpolation of the subsurface data between the borings. Regardless of the thoroughness of a subsurface exploration, there is the possibility that conditions will differ between boring locations, that conditions are not as anticipated by the designers, or that the construction process has modified the soil conditions. Therefore, experienced soil engineers and technicians should evaluate earthwork and foundation construction to verify that the conditions anticipated in design actually exist. Otherwise, we assume no responsibility for construction compliance with the design concepts, specifications or recommendations.

The recommendations contained in this report have been developed on the basis of the previously described project characteristics and subsurface conditions. If project criteria change, we should be permitted to determine if the recommendations should be modified. The findings of such a review will be presented in a supplemental report. Even after completion of a subsurface study, the nature and extent of variation between borings may not become evident until the course of construction. If such variations then become evident, it will be necessary to reevaluate the recommendations of this report after on-site observations of the conditions.

These professional services have been performed, the findings derived, and recommendations prepared in accordance with generally accepted geotechnical engineering principles and practices. This warranty is in lieu of all warranties either expressed or implied. This company is not responsible for the conclusions, opinions or recommendations of others based on these data.

APPENDIX

SOIL TEST BORING PROCEDURES (ASTM D-1586)

The soil test borings were advanced by twisting continuous auger flights into the ground. At selected intervals, soil samples were obtained by driving a standard 1.4 inch I.D., 2.0 inch O.D., split tube sampler into the ground. The sampler was initially seated six inches to penetrate any loose cuttings created in the boring process. The sampler is then driven an additional 12 inches by blows of a 140 pound "hammer" falling 30 inches. The number of blows required to drive the sampler the final foot is designated the Standard Penetration Resistance.

The samples recovered were sealed in glass jars and were transported to the office where they were classified by an engineer in general accordance with the Unified Soil Classification System (USCS).

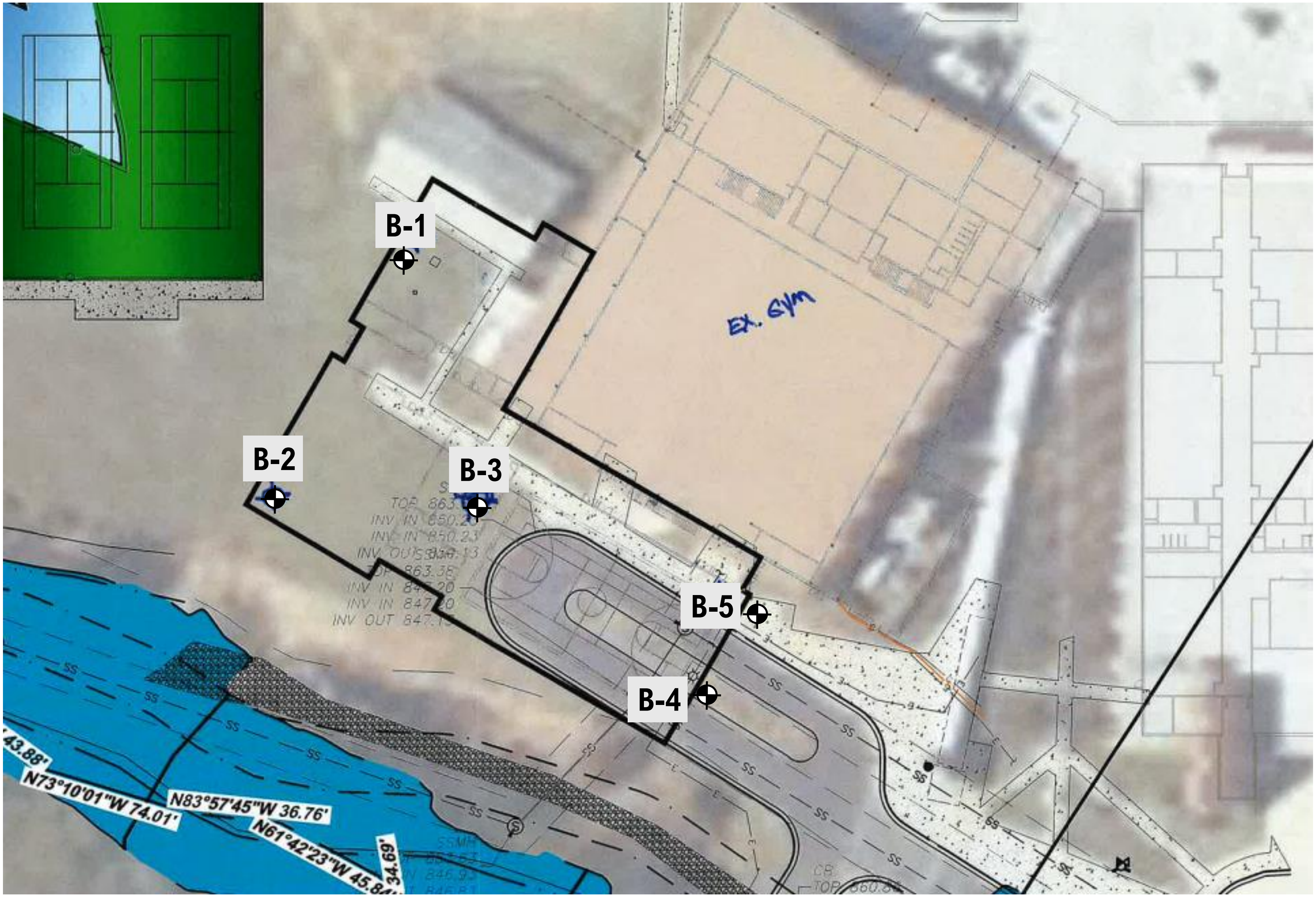
CORRELATION OF STANDARD PENETRATION RESISTANCE WITH RELATIVE COMPACTNESS AND CONSISTENCY

Sand and Gravel

Standard Penetration Resistance Blows / Foot	Relative Compactness
0 - 4	Very Loose
5 - 10	Loose
11 - 30	Medium Dense
31 - 50	Dense
Over 50	Very Dense

Silt and Clay

Standard Penetration Resistance Blows / Foot	Relative Compactness
0 - 1	Very Soft
2 - 4	Soft
5 - 8	Firm
9 - 15	Stiff
16 - 30	Very Stiff
31 - 50	Hard
Over 50	Very Hard



LEGEND	FIGURE 1: SITE AND BORING LOCATION PLAN	ATLAS TECHNICAL CONSULTANTS	DRAWN BY: KB	NOTES:
⊕ SOIL TEST BORING LOCATION	FAYETTE COUNTY HS AUX GYM TIGER TRAIL FAYETTEVILLE, GEORGIA	3000 NORTHFIELD PLACE, SUITE 1100 ROSWELL, GA 30076	APPROVED BY: PROJECT NO.: 18889 DATE: 9/20/24 SCALE: NTS	

SOIL CLASSIFICATION CHART

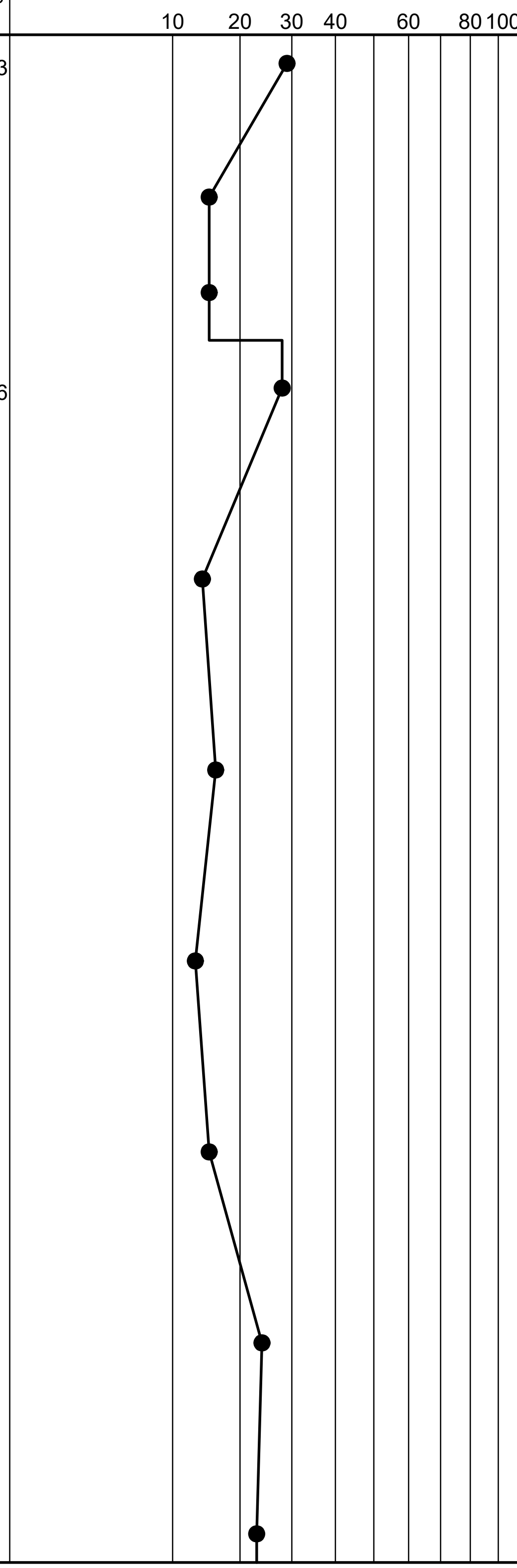
MAJOR DIVISIONS			SYMBOLS		TYPICAL DESCRIPTIONS
			GRAPH	LETTER	
COARSE GRAINED SOILS MORE THAN 50%OF MATERIAL IS LARGER THAN NO. 200 SIEVE SIZE	GRAVEL AND GRAVELLY SOILS MORE THAN 50%OF COARSEFRACTION RETAINED ONNO. 4 SIEVE	CLEAN GRAVELS (LITTLE OR NO FINES)		GW	WELL-GRADED GRAVELS, GRAVEL - SAND MIXTURES, LITTLE OR NO FINES
				GP	POORLY- GRADED GRAVELS, GRAVEL - SAND MIXTURES, LITTLE OR NO FINES
		GRAVELS WITH FINES (APPRECIABLE AMOUNT OF FINES)		GM	SILTY GRAVELS, GRAVEL - SAND - SILT MIXTURES
				GC	CLAYEY GRAVELS, GRAVEL - SAND - CLAY MIXTURES
	SAND AND SANDY SOILS MORE THAN 50%OF COARSEFRACTION PASSING ON NO. 4 SIEVE	CLEAN SANDS (LITTLE OR NO FINES)		SW	WELL-GRADED SANDS, GRAVELLY SANDS, LITTLE OR NO FINES
				SP	POORLY-GRADED SANDS,GRAVELLY SAND, LITTLE OR NO FINES
		SANDS WITH FINES (APPRECIABLE AMOUNT OF FINES)		SM	SILTY SANDS, SAND - SILT MIXTURES
				SC	CLAYEY SANDS, SAND - CLAY MIXTURES
FINE GRAINED SOILS MORE THAN 50% OF MATERIAL IS SMALLER THANNO. 200 SIEVE SIZE	SILTS AND CLAYS	LIQUID LIMIT LESS THAN 50		ML	INORGANIC SILTS AND VERY FINE SANDS, ROCK FLOUR, SILTY OR CLAYEY FINE SANDS OR CLAYEY SILTS WITH SLIGHT PLASTICITY
				CL	INORGANIC CLAYS OF LOW TO MEDIUM PLASTICITY, GRAVELLY CLAYS, SANDY CLAYS, SILTY CLAYS, LEAN CLAYS
				OL	ORGANIC SILTS AND ORGANIC SILTY CLAYS OF LOW PLASTICITY
	SILTS AND CLAYS	LIQUID LIMIT GREATER THAN 50		MH	INORGANIC SILTS, MICACEOUS OR DIATOMACEOUS FINE SAND OR SILTY SOILS
				CH	INORGANIC CLAYS OF HIGH PLASTICITY
				OH	ORGANIC CLAYS OF MEDIUM TO HIGH PLASTICITY, ORGANIC SILTS
ALLUVIUM				PT	ALLUVIUM, PEAT, HUMUS, SWAMP SOILS WITH HIGH ORGANIC CONTENTS
FILL				FILL	MATERIAL PLACED BY MAN

NOTE: DUAL SYMBOLS ARE USED TO INDICATE BORDERLINE SOIL CLASSIFICATIONS

Fayette County High School

DEPTH (FT)	DESCRIPTION	ELEV. 862	BLOW COUNTS	N VALUE
	TOPSOIL/ROOT ZONE: 4 inches			
	FILL: Medium dense brown tan orange clayey silty fine to medium SAND (SM), trace mica and rock fragments		13-16-13	29
5		857	7-7-8	15
			7-8-7	15
	RESIDUUM: Very stiff orange red brown fine to medium sandy clayey SILT (ML), trace mica			
10		852	10-12-16	28
	Medium dense brown orange gray silty fine to medium SAND (SM), micaceous			
15		847	6-7-7	14
20		842	7-8-8	16 ▽
	Medium dense brown gray orange silty fine to medium SAND (SM), micaceous, wet			
25		837	5-5-8	13
30		832	5-7-8	15
	Medium dense gray white brown silty medium to fine SAND (SM)			
35		827	5-7-17	24
40		822	6-9-14	23

PENETRATION (BLOWS PER FOOT)



SOIL BORING RECORD

BORING NUMBER	B-1
DATE DRILLED	9/21/2024
PROJECT NUMBER	18889
PAGE	1 of 2

▽ Groundwater level at time of boring
▼ Groundwater level - 24 hrs
C Caved depth - 24 hrs
☒ Standard penetration test
■ Undisturbed sample

Fayette County High School

DEPTH (FT)	DESCRIPTION	ELEV. 822	BLOW COUNTS	PENETRATION (BLOWS PER FOOT) 10 20 30 40 60 80 100	N VALUE
	Medium dense gray white brown silty medium to fine SAND (SM) *(continued)*				
45	PARTIALLY WEATHERED ROCK: Sampled as very dense gray white silty coarse to fine SAND (SM)	817	50/6"	>>	50/6"
	Auger refusal at 46.5 feet				
50					
55					
60					
65					
70					
75					
80					

Groundwater level at time of boring
Groundwater level - 24 hrs
Caved depth - 24 hrs
Standard penetration test
Undisturbed sample

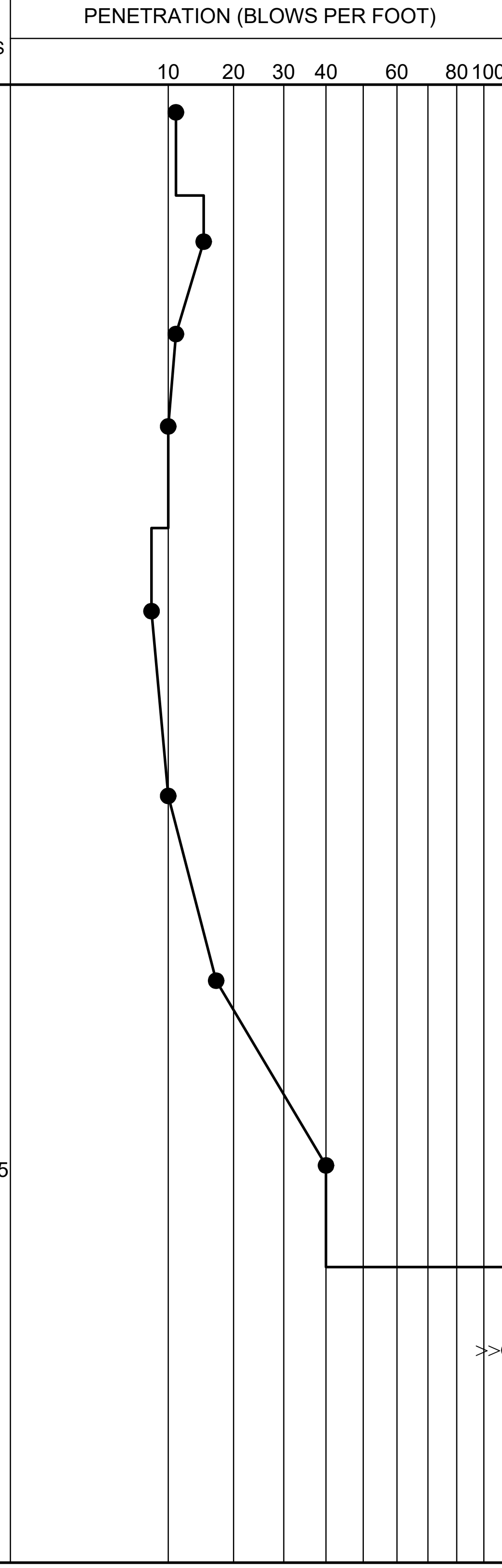
SOIL BORING RECORD

BORING NUMBER	B-1
DATE DRILLED	9/21/2024
PROJECT NUMBER	18889
PAGE	2 of 2

ATLAS

Fayette County High School

DEPTH (FT)	DESCRIPTION	ELEV. 859	BLOW COUNTS	N VALUE
0	TOPSOIL/ROOT ZONE: 6 inches			
	FILL: Stiff brown orange red clayey sandy SILT (ML), trace mica		5-5-6	11
3	ALLUVIUM: Stiff brown gray black clayey sandy SILT (ML), organics			
5		854	6-7-8	15
			5-6-5	11
8	Loose gray tan silty clayey medium to fine SAND (SC), wet			
10		849	5-5-5	10
12	RESIDUUM: Firm orange red brown clayey fine sandy SILT (ML), trace mica			
15		844	2-4-4	8
18	Loose to medium dense brown orange tan silty medium to fine SAND (SM), micaceous, wet			
20		839	5-4-6	10
25		834	6-8-9	17
26	Dense gray tan white silty coarse to fine SAND (SM), trace mica			
30		829	10-15-25	40
32	PARTIALLY WEATHERED ROCK: No sample recovered			
35		824	50/0"	50/0"
	Auger refusal at 35 feet			
40				



SOIL BORING RECORD

BORING NUMBER	B-2
DATE DRILLED	9/21/2024
PROJECT NUMBER	18889
PAGE	1 of 1

∇ Groundwater level at time of boring

▼ Groundwater level - 24 hrs

C Caved depth - 24 hrs

⊠ Standard penetration test

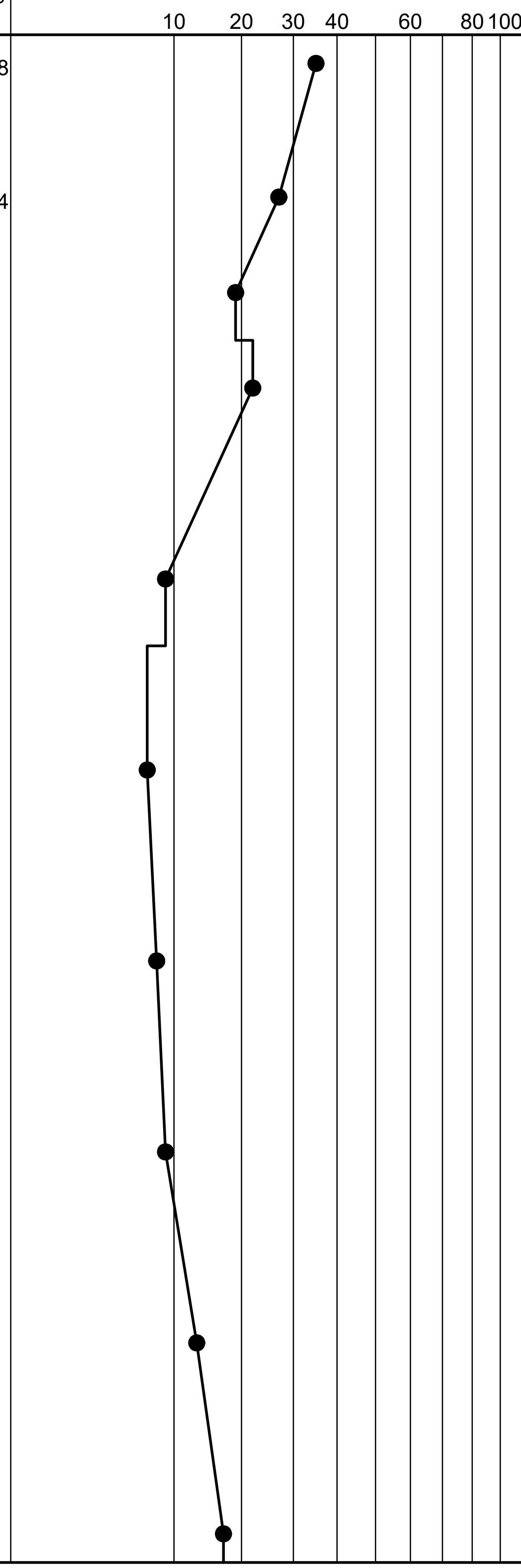
■ Undisturbed sample

ATLAS

Fayette County High School

DEPTH (FT)	DESCRIPTION	ELEV. 862	BLOW COUNTS	N VALUE
0	TOPSOIL/ROOT ZONE: 4 inches			
1	FILL: Dense to medium dense brown orange clayey silty medium to fine SAND (SM)		14-17-18	35
5		857	12-13-14	27
7.5			6-9-10	19
8	ALLUVIUM: Very stiff red brown medium to fine sandy clayey SILT (ML-MH), wet			
10		852	5-7-15	22
12	Loose brown clayey silty medium to fine SAND (SM)			
15		847	3-4-5	9
16	RESIDUUM: Loose white gray silty medium to fine SAND (SM), micaceous, wet			
20		842	2-2-5	7
22	Firm to stiff brown orange black fine to medium sandy SILT (ML), micaceous			
25		837	2-4-4	8
30		832	4-4-5	9
32	Medium dense brown gray black silty fine to medium SAND (SM), micaceous			
35		827	5-6-7	13
40		822	5-8-9	17

PENETRATION (BLOWS PER FOOT)



SOIL BORING RECORD

BORING NUMBER	B-3
DATE DRILLED	9/21/2024
PROJECT NUMBER	18889
PAGE	1 of 2

▽ Groundwater level at time of boring
▼ Groundwater level - 24 hrs
C Caved depth - 24 hrs
☒ Standard penetration test
▮ Undisturbed sample

Fayette County High School

DEPTH (FT)	DESCRIPTION	ELEV. 822	BLOW COUNTS	PENETRATION (BLOWS PER FOOT) 10 20 30 40 60 80 100	N VALUE
	Medium dense brown gray black silty fine to medium SAND (SM), micaceous *(continued)*				
	PARTIALLY WEATHERED ROCK: Sampled as very dense gray black tan silty medium to fine SAND (SM)				
45		817	50/6"	>>	50/6"
50		812	50/0"	>>	50/0"
	Boring terminated at 50 feet				
55					
60					
65					
70					
75					
80					

SOIL BORING RECORD

BORING NUMBER	B-3
DATE DRILLED	9/21/2024
PROJECT NUMBER	18889
PAGE	2 of 2

Groundwater level at time of boring
Groundwater level - 24 hrs
Caved depth - 24 hrs
Standard penetration test
Undisturbed sample

Fayette County High School

DEPTH (FT)	DESCRIPTION	ELEV. 865	BLOW COUNTS	N VALUE
0	TOPSOIL/ROOT ZONE: 4 inches			
	FILL: Medium dense brown orange clayey silty fine to medium SAND (SM)		12-13-16	29
3	Very stiff brown orange red clayey medium to fine sandy SILT (ML), trace mica			
5		860	8-10-15	25
			7-8-8	16
8	Medium dense brown tan orange silty fine to medium SAND (SM)			
10		855	5-6-6	12
15		850	6-8-8	16
15	RESIDUUM: Medium dense brown tan gray silty medium to fine SAND (SM), micaceous			
20		845	5-5-9	14
22	Medium dense brown orange black silty fine to medium SAND (SM), micaceous, wet			
25		840	5-5-8	13
30		835	5-7-9	16
32	PARTIALLY WEATHERED ROCK: Sampled as very dense tan black white silty medium to fine SAND (SM), micaceous			
35		830	12-50/3"	50/3"
36.5	Auger refusal at 36.5 feet			
40				

PENETRATION (BLOWS PER FOOT): 10 20 30 40 60 80 100

SOIL BORING RECORD

BORING NUMBER	B-4
DATE DRILLED	9/21/2024
PROJECT NUMBER	18889
PAGE	1 of 1

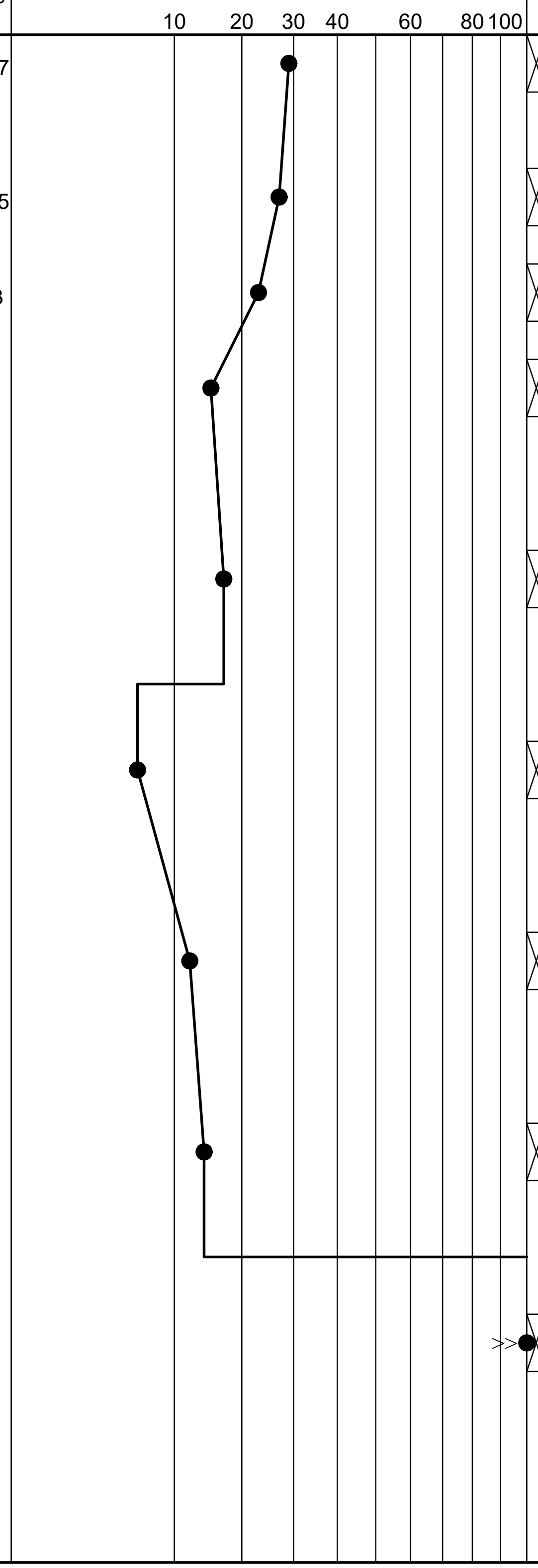
- ▽ Groundwater level at time of boring
- ▼ Groundwater level - 24 hrs
- C Caved depth - 24 hrs
- ☒ Standard penetration test
- ▮ Undisturbed sample

ATLAS

Fayette County High School

DEPTH (FT)	DESCRIPTION	ELEV. 866	BLOW COUNTS	N VALUE
0	TOPSOIL/ROOT ZONE: 2 inches			
0–6	FILL: Very stiff brown orange clayey fine to medium sandy SILT (ML), trace mica		10-12-17	29
5		861	10-12-15	27
6–11	Very stiff to stiff brown red sandy clayey SILT (ML-MH)		9-10-13	23
10		856	5-5-10	15
11–17	POSSIBLE FILL/ALLUVIUM: Medium dense brown gray black clayey silty medium to fine SAND (SM), trace organics			
15		851	5-7-10	17
17–22	RESIDUUM: Loose brown tan black silty fine to medium SAND (SM), micaceous, wet			
20		846	2-2-4	6
22–32	Medium dense brown black tan silty medium to fine SAND (SM), micaceous			
25		841	3-5-7	12
30		836	5-6-8	14
32–33.5	PARTIALLY WEATHERED ROCK: No sample recovered			
33.5	Auger refusal at 33.5 feet			
35			50/0"	50/0"
40				

PENETRATION (BLOWS PER FOOT)



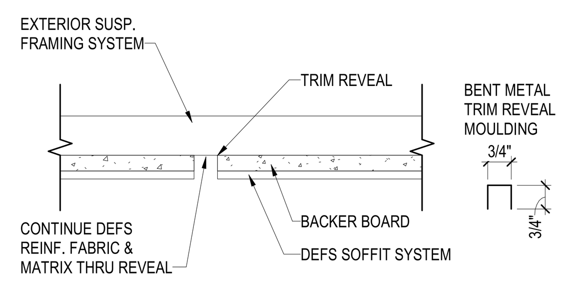
∇ Groundwater level at time of boring
▼ Groundwater level - 24 hrs
C Caved depth - 24 hrs
☒ Standard penetration test
▮ Undisturbed sample

SOIL BORING RECORD

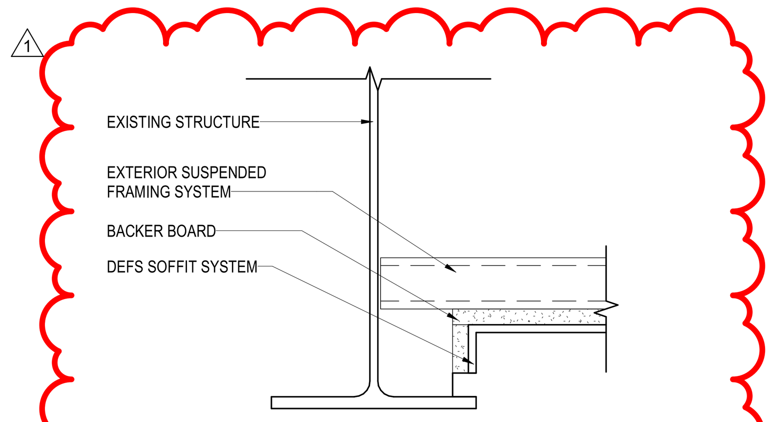
BORING NUMBER	B-5
DATE DRILLED	9/21/2024
PROJECT NUMBER	18889
PAGE	1 of 1

CONTRACT DRAWINGS

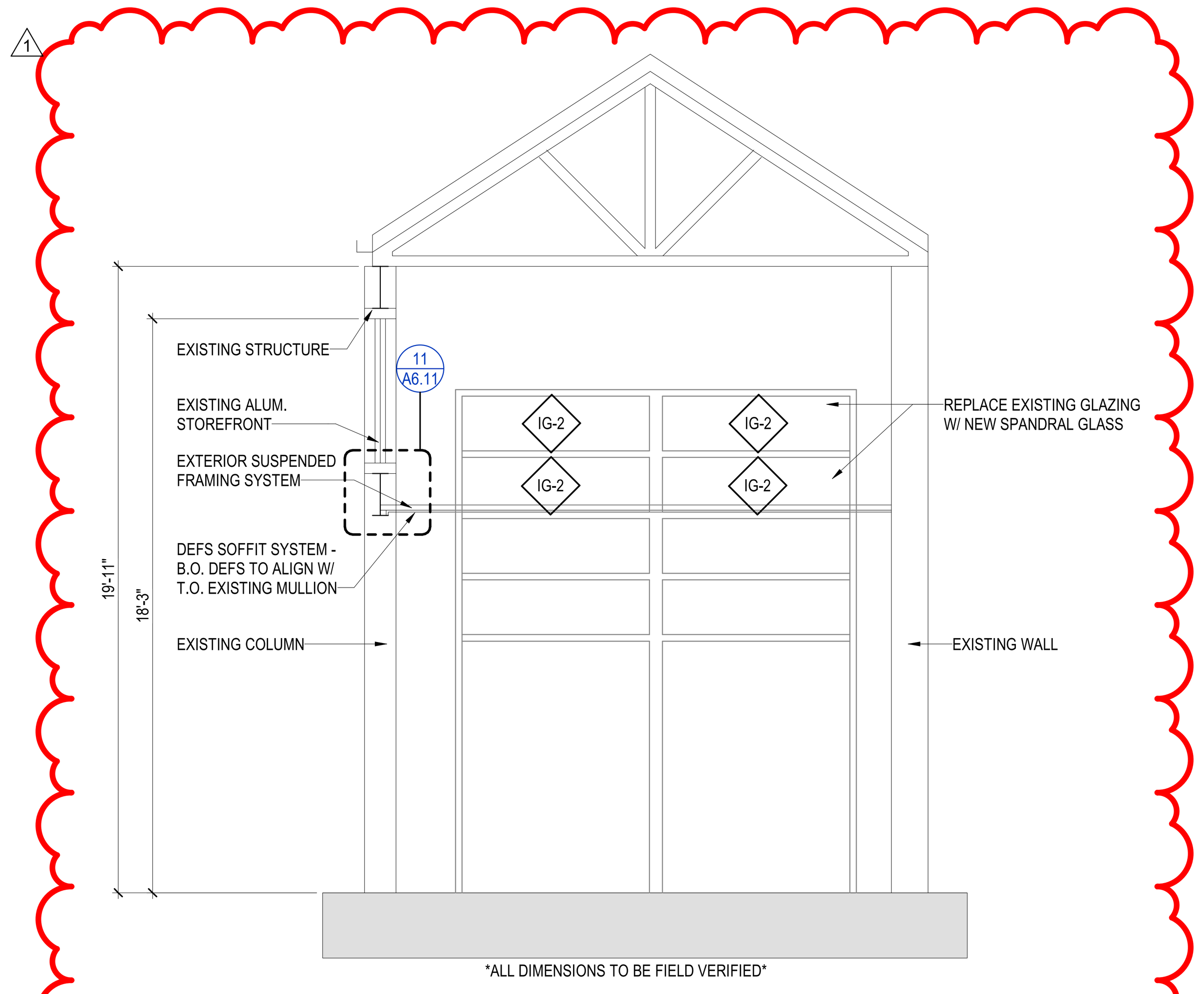
MISC. DETAILS
A6.11
FOR BLUEBEAM LABELING/OCR:



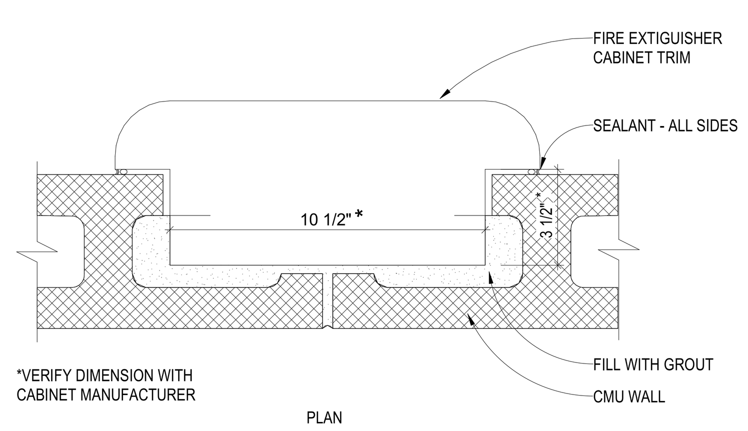
12 A6.11 TYP. DEFS AT SOFFIT REVEAL
3" = 1'-0"

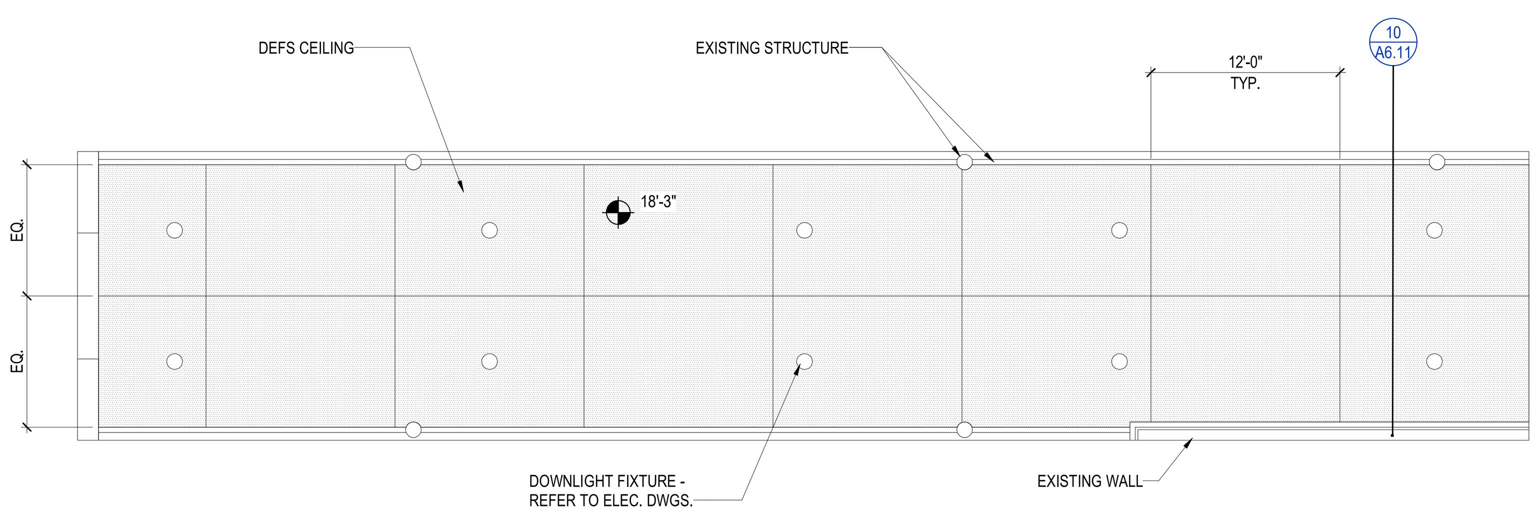


11 A6.11 DETAIL AT FRONT CANOPY
3" = 1'-0"

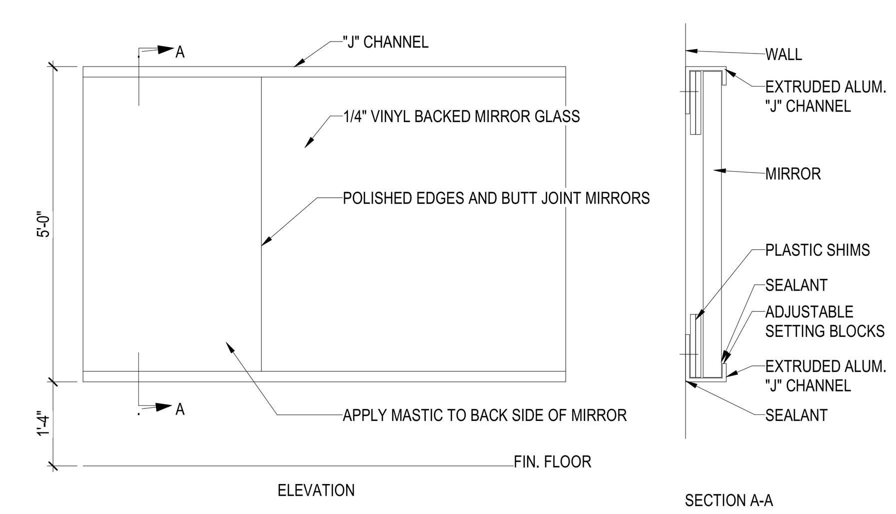


10 A6.11 FRONT CANOPY SECTION
1/4" = 1'-0"

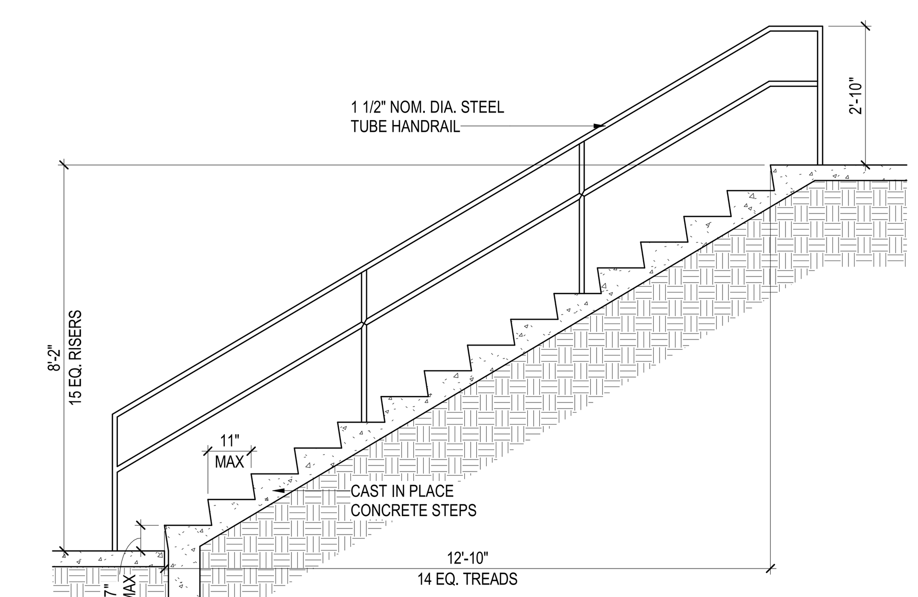




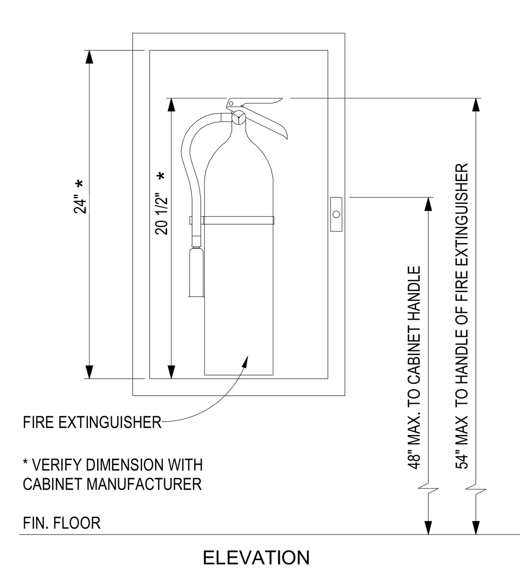
9 A6.11 FRONT CANOPY RCP
1/8" = 1'-0"



8 A6.11 MIRROR DETAIL
1/2" = 1'-0"

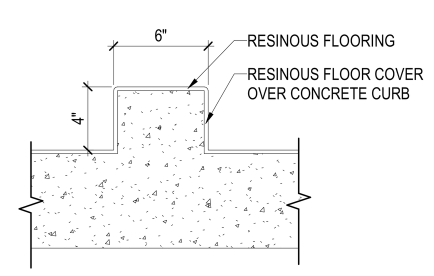


7 A6.11 STAIR SECTION
3/8" = 1'-0"

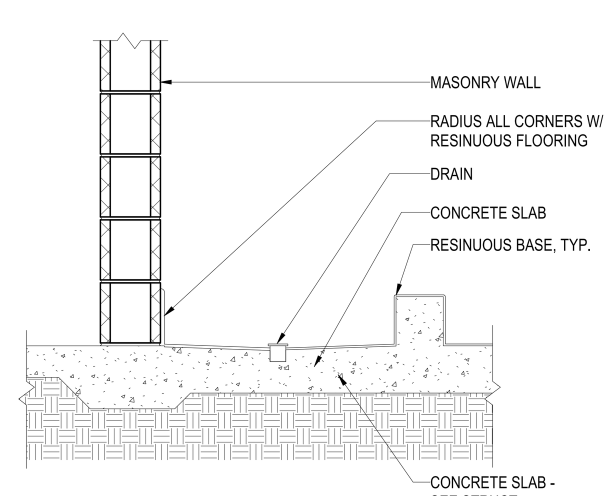


NOTE:
1. IN FIRE RATED WALLS PROVIDE FIRE RATED CABINETS TO MATCH RATING OF WALL

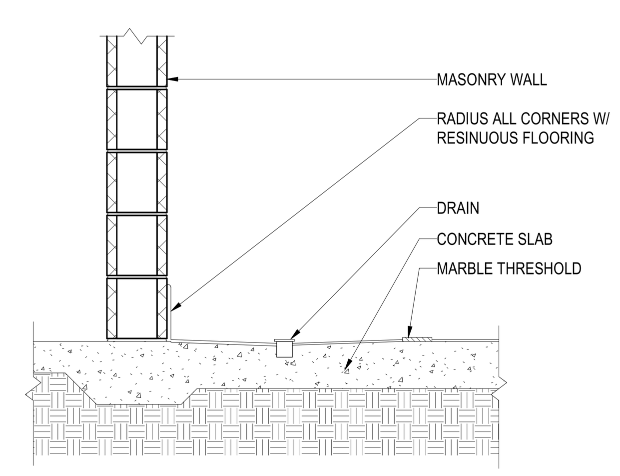
6 A6.11 FIRE EXTINGUISHER DETAIL
3" = 1'-0"



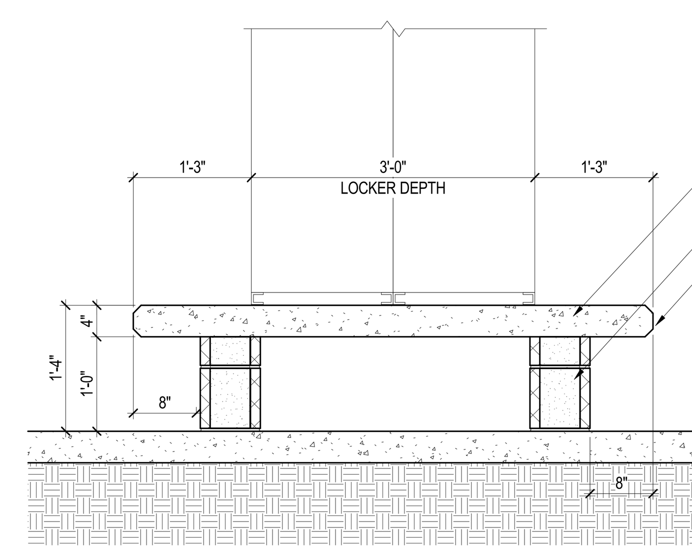
5 A6.11 SECTION DETAIL - CURB
1 1/2" = 1'-0"



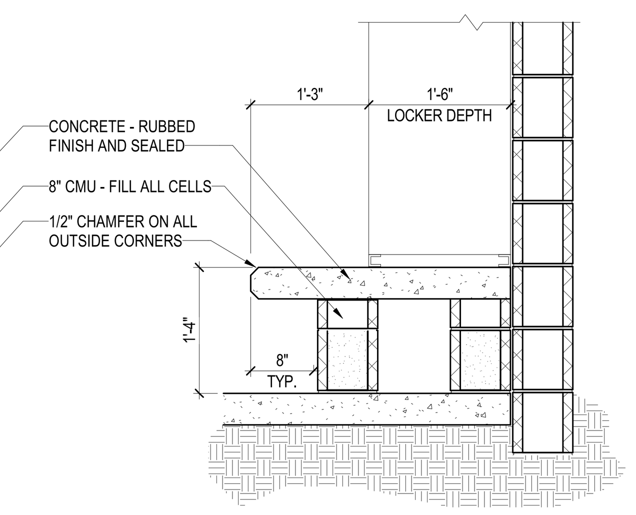
4 A6.11 SECTION DETAIL
3/4" = 1'-0"



3 A6.11 SECTION DETAIL
3/4" = 1'-0"



2 A6.11 LOCKER SECTION DETAIL
3/4" = 1'-0"



1 A6.11 LOCKER SECTION DETAIL
3/4" = 1'-0"

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ARCHITECT
MSSA-PBK, Inc.
www.mssa-pbk.com

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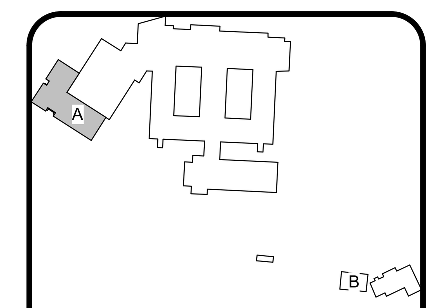
Architect Registration No. 8279

FAYETTE COUNTY HIGH SCHOOL - AUXILIARY GYM ADDITION and FIELD HOUSE IMPROVEMENTS

1 TIGER TRAIL, FAYETTEVILLE, GA 30214



KEY PLAN PLAN NORTH TRUE NORTH

DATE	PROJECT NUMBER
NOVEMBER 13, 2024	202311

REVISIONS / ISSUANCES

No.	Description	Date
1	ADDENDA #1	02/10/25

MISC. DETAILS

A6.11

File Path: Autodesk Docs://Fayette County Schools_202311_HS Auxiliary Gym Addition/202311 - Fayette County High School Auxilary Gym.rvt

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